ORDINANCE NO. _____

An ordinance amending Title 30 – Residential Code – of the Los Angeles County Code, by adopting and incorporating by reference the 2025 California Residential Code, with certain changes and modifications.

The Board of Supervisors of the County of Los Angeles ordains as follows:

SECTION 1. Chapters 2 through 10, Chapter 44, and Appendices AH, AQ, AS, and AZ, of Title 30 of the Los Angeles County Code, which incorporate by reference, and modify, portions of the 2022 California Residential Code, are hereby repealed.

SECTION 2. Chapter 1 is hereby amended to read as follows:

R100 ADOPTION AND INCORPORATION BY REFERENCE

Except as hereinafter changed or modified, Sections 102 through 119 of Chapter 1, Section 1206 of Chapter 12, and Chapters 67, 68, 69, 98, 99, and Appendix J of Title 26 of the Los Angeles County Code are adopted and incorporated by reference into this Title 30 as if fully set forth below, and shall be known as Sections 102 through 119 of Chapter 1, Section 1206 of Chapter 12, and Chapters 67, 68, 69, 98, 99, and Appendix J of Title 30 of the Los Angeles County Code.

Except as hereinafter changed or modified, Chapters 2 through 10, Chapter 44, and Appendices ~~AH, AQ, AS, and AZ~~BB, BF, BJ, and CJ, of that certain code known and designated as the ~~2022~~2025 California Residential Code as published by the California Building Standards Commission are adopted and incorporated by reference into this Title 30 as if fully set forth below, and shall be known as Chapters 2 through 10,

Chapter 44, and Appendices ~~AH, AQ, AS, and AZ~~BB, BF, BJ, and CJ, of Title 30 of the Los Angeles County Code. A copy of the ~~2022~~2025 California Residential Code shall be at all times maintained by the Building Official for use and examination by the public.

R101 TITLE, PURPOSE, AND INTENT

. . .

R101.3 Scope.

The provisions of this Code shall apply to the construction, alteration, movement, enlargement, replacement, repair, equipment, use and occupancy, location, removal, demolition, and grading of detached one- and two-family dwellings and townhouses not more than three stories above grade plane in height with a separate means of egress and their accessory structures that are located within the unincorporated territory of the County of Los Angeles and to such work or use by the County of Los Angeles in any incorporated city.

Exceptions:

. . .

2. Owner-occupied lodging houses with five or fewer guestrooms shall be permitted to be constructed in accordance with the Los Angeles County Residential Code for one- and two-family dwellings when equipped with a fire sprinkler system in accordance with Section ~~R313~~R309.

. . .

SECTION 3. Section R301.1.3.2 is hereby amended to read as follows:

R301.1.3.2 Woodframe structures ~~greater than two-stories~~.

The ~~b~~Building ~~o~~Official shall require construction documents to be approved and stamped by a California licensed architect or engineer for all dwellings of woodframe construction more than two stories and basement in height located in Seismic Design Category A, B, or C. Notwithstanding other sections of law, the law establishing these provisions is found in Business and Professions Code Sections 5537 and 6737.1.

The Building Official shall require construction documents to be approved and stamped by a California licensed architect or engineer for all structural aspects of dwellings of woodframe construction more than one story in height or with a basement located in Seismic Design Category D_0, D_1, or D_2 or E.

SECTION 4. Section R301.1.5 is hereby added to read as follows:

R301.1.5 Seismic design provisions for buildings constructed on or into slopes steeper than one unit vertical in three units horizontal (33.3 percent slope).

The design and construction of new buildings and additions to existing buildings when constructed on or into slopes steeper than one unit vertical in three units horizontal (33.3 percent slope) shall comply with Section 1613.6 of the Los Angeles County Building Code.

SECTION 5. Section R301.2 is hereby amended to read as follows:

R301.2 Climatic and geographic design criteria.

Buildings shall be constructed in accordance with the provisions of this ~~c~~Code as limited by the provisions of this ~~s~~Section. ~~Additional criteria shall be established by the~~

~~local jurisdiction and set forth~~Consult with the Building Official regarding additional criteria in Table R301.2.

SECTION 6. Section R301.2.2.6 is hereby amended to read as follows:

R301.2.2.6 Irregular buildings.

. . .

1. **Shear wall or braced wall offsets out of plane.** Conditions where exterior shear wall lines or braced wall panels are not in one plane vertically from the foundation to the uppermost story in which they are required.

~~**Exception:** For wood light-frame construction, floors with cantilevers or setbacks not exceeding four times the nominal depth of the wood floor joists are permitted to support braced wall panels that are out of plane with braced wall panels below provided that all of the following are satisfied:~~

~~1. Floor joists are nominal 2 inches by 10 inches (51 mm by 254 mm) or larger and spaced not more than 16 inches (406 mm) on center.~~

~~2. The ratio of the back span to the cantilever is not less than 2 to 1.~~

~~3. Floor joists at ends of braced wall panels are doubled.~~

~~4. For wood-frame construction, a continuous rim joist is connected to ends of cantilever joists. Where spliced, the rim joists shall be spliced using a galvanized metal tie not less than 0.058 inch (1.5 mm) (16 gage) and 11/2 inches (38 mm) wide fastened with six 16d nails on each side of the splice; or a block of the same size as the rim joist and of sufficient length to fit securely between the joist space at which the splice occurs, fastened with eight 16d nails on each side of the splice.~~

~~5. Gravity loads carried at the end of cantilevered joists are limited to uniform wall and roof loads and the reactions from headers having a span of 8 feet (2438 mm) or less.~~

2. **Lateral support of roofs and floors.** Conditions where a section of floor or roof is not laterally supported by shear walls or braced wall lines on all edges.

Exception: Portions of floors that do not support shear walls, braced wall panels above, or roofs shall be permitted to extend not more than 6 feet (1829 mm) beyond a shear wall or braced wall line.

3. **Shear wall or braced wall offsets in plane.** Conditions where the end of a braced wall panel occurs over an opening in the wall below ~~and extends more than 1 foot (305 mm) horizontally past the edge of the opening. This provision is applicable to shear walls and braced wall panels offset in plane and to braced wall panels offset out of plane in accordance with the exception to Item 1~~.

~~**Exception:** For wood light-frame wall construction, one end of a braced wall panel shall be permitted to extend more than 1 foot (305 mm) over an opening not more than 8 feet (2438 mm) in width in the wall below provided that the opening includes a header in accordance with all of the following:~~

~~1. The building width, loading condition and framing member species limitations of Table R602.7(1) shall apply.~~

~~2. The header is composed of:~~

~~2.1 Not less than one 2x12 or two 2x10 for an opening not more than 4 feet (1219 mm) wide.~~

~~2.2. Not less than two 2x12 or three 2x10 for an opening not more than 6 feet (1829 mm) in width.~~

~~2.3. Not less than three 2x12 or four 2x10 for an opening not more than 8 feet (2438 mm) in width.~~

~~3. The entire length of the braced wall panel does not occur over an opening in the wall below.~~

4. **Floor and roof opening.** Conditions where an opening in a floor or roof exceeds the lesser of 12 feet (3658 mm) or 50 percent of the least floor or roof dimension.

5. **Floor level offset.** Conditions where portions of a floor level are vertically offset.

~~**Exceptions:**~~

~~1. Framing supported directly by continuous foundations at the perimeter of the building.~~

~~2. For wood light-frame construction, floors shall be permitted to be vertically offset where the floor framing is lapped or tied together as required by section R502.6.1.~~

. . .

SECTION 7. Section R301.2.2.11 is hereby added to read as follows:

R301.2.2.11 Anchorage of mechanical, electrical, or plumbing components and equipment.

Mechanical, electrical, or plumbing components and equipment shall be anchored to the structure. Anchorage of the components and equipment shall be

designed to resist loads in accordance with the Los Angeles County Building Code and ASCE 7, except where the component is positively attached to the structure and flexible connections are provided between the component and associated ductwork, piping, and conduit; and either:

1. The component weighs 400 pounds (1,780 N) or less and has a center of mass located 4 feet (1.22 m) or less above the supporting structure; or

2. The component weighs 20 pounds (89N) or less or, in the case of a distributed system, 5 pounds per foot (73 N/m) or less.

SECTION 8. Table R302.1(2) is hereby amended as follows:

TABLE R302.1(2)—
EXTERIOR WALLS—DWELLINGS, TOWNHOUSES *AND ACCESSORY BUILDINGS WITH AUTOMATIC RESIDENTIAL FIRE SPRINKLER PROTECTION*

EXTERIOR WALL ELEMENT		MINIMUM FIRE-RESISTANCE RATING	MINIMUM FIRE SEPARATION DISTANCE
Walls	Fire-resistance rated	1 hour—tested in accordance with ASTM E119, UL 263 or Section 703.2.2 of the *California Building Code* with exposure from the outside	0 feet
	Not fire-resistance rated	0 hours	3 feet[a]
Projections	Not allowed	NA	< 2 feet
	Fire-resistance rated	1 hour on the underside, or heavy timber, or fire-retardant-treated wood[b, c]	2 feet[a]
	Not fire-resistance rated	0 hours	3 feet
Openings in walls	Not allowed	NA	< 3 feet
	Unlimited	0 hours	3 feet[a]
Penetrations	All	Comply with Section R302.4	< 3 feet
		None required	3 feet[a]

For SI: 1 foot = 304.8 mm

NA = Not Applicable.

a. Reserved.~~For residential subdivisions where all dwellings and townhouses are equipped throughout with an automatic sprinkler system installed in accordance with Section R309, the fire separation distance for exterior walls not fire-resistance rated and for fire-resistance-rated projections shall be permitted to be reduced to 0 feet, and unlimited unprotected openings and penetrations shall be permitted, where the adjoining lot provides an open setback yard that is 6 feet or more in width on the opposite side of the property line.~~

. . .

SECTION 9. Section R337.1 is hereby added as follows:

R337.1 Modifications to the California Wildland-Urban Interface Code.

Chapter 5 of the California Wildland-Urban Interface Code shall be modified as indicated in Sections 337.1.1 through 337.1.7 and shall apply to all buildings and structures as provided in this section.

337.1.1 California Wildland-Urban Interface Code, Section 501.1.

Modify the California Wildland-Urban Interface Code Section 501.1 to read as follows:

501.1 Scope and Application.

New b~~B~~uildings and structures in a wildland-urban interface area shall be constructed in accordance with the ~~California~~ Residential Code and this Code. Additions, alterations, or repairs made to existing buildings erected, constructed, or moved within a Fire Hazard Severity Zone or Wildland-Urban Interface area shall be constructed in accordance with the Residential Code and this code. New buildings for which an application for a building permit is submitted on or after July 1, 2008, and any additions, alterations, or repairs made to existing buildings for which an application for a

building permit is submitted on or after January 1, 2026, located in any Fire Hazard Severity Zone or Wildland-Urban Interface area shall comply with all sections of the Residential Code and this Code.

Exceptions:

1. Group U accessory structures not exceeding 120 square feet (11m^2) in floor area where located not less than 50 feet (15 240 mm) from applicable buildings.
2. Group U agricultural buildings not less than 50 feet (15 240 mm) from applicable buildings.

501.1.1 Alternates for materials, design, tests, and methods of construction.

The Building Official is permitted to modify the provisions of this Chapter for site-specific conditions in accordance with Chapter 1, Section 104.2.2.

337.1.2 California Wildland-Urban Interface Code, Section 503.1

Modify the California Wildland-Urban Interface Code Section 503.1 to read as follows:

503.1 General.

Buildings and structures hereafter constructed, modified or relocated into or within wildland-urban interface areas shall meet the construction requirements in accordance with Chapter 5. Materials required to be ignition-resistant building materials shall comply with the requirements of Section 503.2.

Exceptions:

1. New accessory buildings and miscellaneous structures complying with Section 504.11.
2. Reserved. ~~Additions to and remodels of buildings originally constructed prior to July 1, 2008.~~
3. Group C occupancy special buildings conforming to the limitations specified in Section 450.4.1 of the ~~California~~ Building Code.

337.1.3 California Wildland-Urban Interface Code, Section 503.2.3

Modify the California Wildland-Urban Interface Code Section 503.2.3 to read as follows:

503.2.3 Reserved. ~~Fire-retardant-treated wood roof coverings.~~

~~Roof assemblies containing fire-retardant-treated wood shingles and shakes shall comply with the requirements of Section 1505.6 of the California Building Code and shall be classified as Class A roof assemblies as required in Section 1505.2 of the California Building Code.~~

337.1.4 California Wildland-Urban Interface Code, Section 503.3.1

Modify the California Wildland-Urban Interface Code Section 503.3.1 to read as follows:

503.3.1 Qualification by testing.

Material and material assemblies tested in accordance with the requirements of Section 503 shall be accepted for use when the results and conditions of those tests

are met. Product evaluation testing of material and material assemblies shall be approved or listed by the State Fire Marshal, the Building Official, or identified in a current report issued by an approved agency.

337.1.5 California Wildland-Urban Interface Code, Section 503.3.4

Modify the California Wildland-Urban Interface Code Section 503.3.4 to read as follows:

503.3.4 Reserved. ~~Fire-retardant-treated wood shingles and shakes.~~

~~Fire-retardant-treated wood shingles and shakes shall be approved and listed by the State Fire Marshal in accordance with Section 208(c), Title 19 California Code of Regulations.~~

337.1.6 California Wildland-Urban Interface Code, Section 504.2.1

Modify the California Wildland-Urban Interface Code Section 504.2.1 to read as follows:

504.2.1 Roof covering voids.

Where there is a void under the roof covering it shall comply with Section 504.2.1.1 or 504.2.1.2. Roof coverings shall be Class A as specified in the Building Code. Wood shingles and wood shakes are prohibited in any Fire Hazard Severity Zone regardless of classification.

337.1.7 California Wildland-Urban Interface Code, Section 504.5.2

Modify the California Wildland-Urban Interface Code Section 504.5.2 to read as follows:

504.5.2 Exterior wall coverings.

Exterior wall coverings shall comply with one or more of the following requirements:

1. Noncombustible material.

2. Ignition-resistant building material labeled for exterior use.

3. Fire-retardant treated wood labeled for exterior use and complying with the requirements of Section 2303.2 of the ~~California~~ Building Code.

4. Reserved. ~~Fire-retardant-treated wood shingles and shakes which have been qualified in accordance with Section 1505.6 of the California Building Code for use as "Class B" roof covering shall be an acceptable alternative wall covering material where installed over solid sheathing.~~

Exception:

Exterior wall coverings which are a component of an approved wall assembly complying with Section 504.5.

SECTION 10. Section R401.1 is hereby amended to read as follows:

R401.1 Application.

. . .

Wood foundations in Seismic Design Category D_0, D_1, or D_2 shall ~~be designed in accordance with accepted engineering practice~~not be permitted.

Exception: In non-occupied, single-story, detached storage sheds and similar uses other than carport or garage, provided the gross floor area does not exceed 200 square feet, the plate height does not exceed 12 feet in height above the grade plane at any point, and the maximum roof projection does not exceed 24 inches.

SECTION 11. Section R403.1.2 is hereby amended to read as follows:

R403.1.2 Continuous footing in seismic design categories D_0, D_1 and D_2.

Exterior walls and required interior braced wall panels of buildings located in Seismic Design Categories D_0, D_1 and D_2 shall be supported by continuous solid or fully grouted masonry or concrete footings ~~in accordance with Table R403.1.2~~. ~~Other footing materials or systems shall be designed in accordance with accepted engineering practice.~~

SECTION 12. Section R403.1.3.6 is hereby amended to read as follows:

R403.1.3.6 Isolated concrete footings.

In detached one- and two-family dwellings located in Seismic Design Category A, B, or C that are three stories or less in height and constructed with stud bearing walls, isolated plain concrete footings supporting columns or pedestals are permitted.

SECTION 13. Section R403.1.5 is hereby amended to read as follows:

R403.1.5 Slope.

The top surface of footings shall be level. The bottom surface of footings shall not have a slope exceeding 1 unit vertical in 10 units horizontal (10-percent slope). Footings shall be stepped where it is necessary to change the elevation of the top surface of the footings or where the slope of the bottom surface of the footings will exceed one unit vertical in 10 units horizontal (10-percent slope).

For structures located in Seismic Design Category D_0, D_1, or D_2, stepped footings shall be reinforced with two No. 4 reinforcing bars. Two bars shall be located at the top and bottom of the footings as shown in Figure R403.1.5.

SECTION 14. Figure R403.1.5 is hereby added to read as follows:

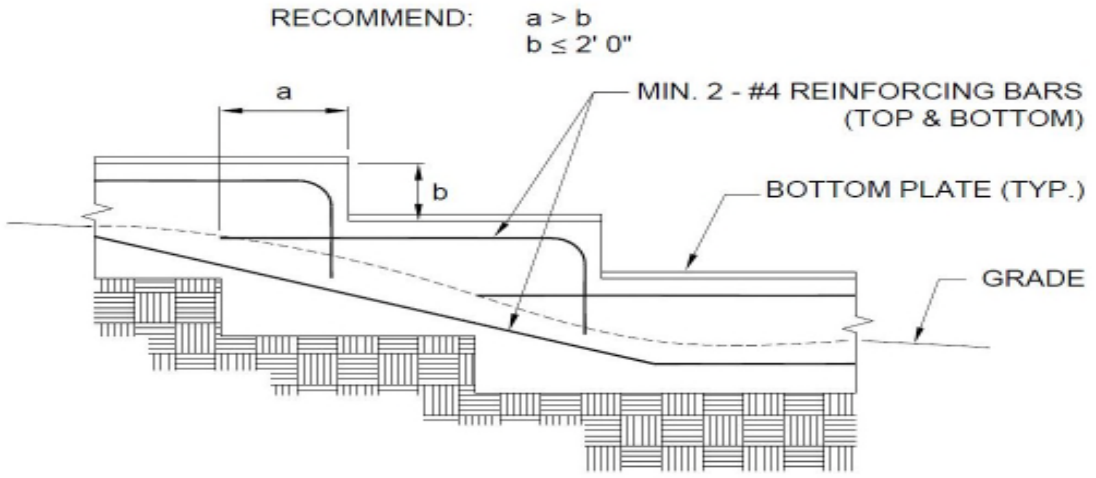


FIGURE R403.1.5
STEPPED FOOTING

SECTION 15. Section R404.2 is hereby amended to read as follows:

R404.2 Wood foundation walls.

Wood foundation walls shall be constructed in accordance with the provisions of Sections R404.2.1 through R404.2.6 and with the details shown in Figures R403.1(2)

and R403.1(3). Wood foundation walls shall not be used for structures located in Seismic Design Category D_0, D_1, or D_2.

SECTION 16. Section R501.2 is hereby amended to read as follows:

R501.2 Requirements.

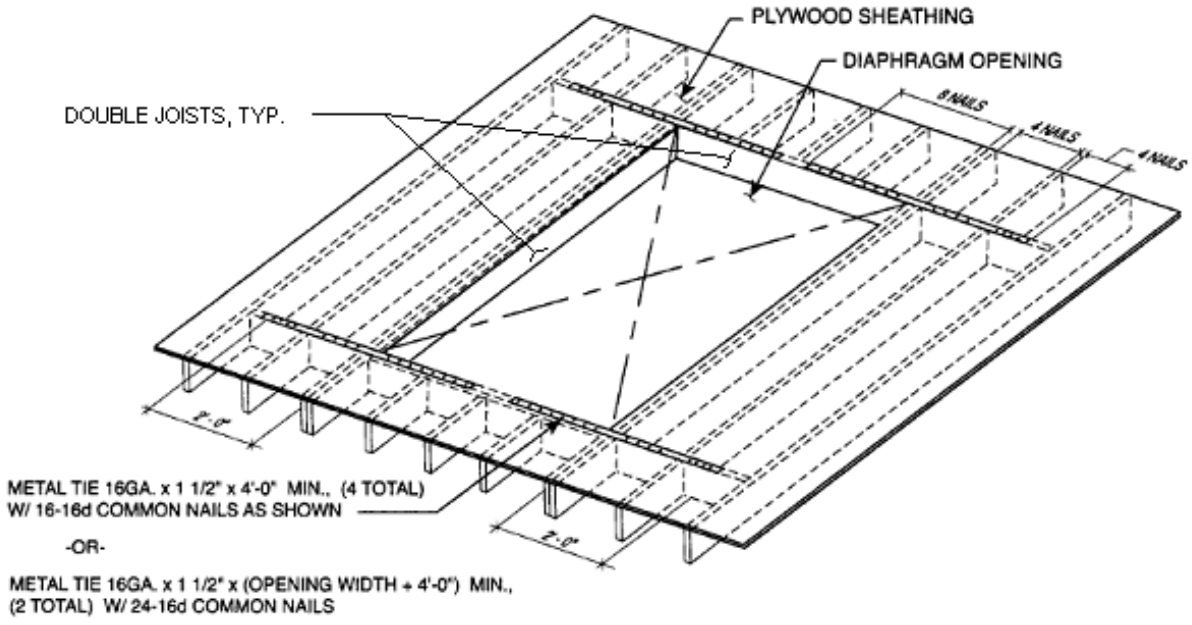
Floor construction shall be capable of accommodating all loads in accordance with Section R301 and of transmitting the resulting loads to the supporting structural elements. Mechanical or plumbing fixtures and equipment shall be attached or anchored to the structure in accordance with Section R301.2.2.11.

SECTION 17. Section R503.2.4 is hereby added to read as follows:

R503.2.4 Openings in horizontal diaphragms.

Openings in horizontal diaphragms with a dimension perpendicular to the joist that is greater than 4 feet (1.2 m) shall be constructed in accordance with Figure R503.2.4.

SECTION 18. Figure R503.2.4 is hereby added to read as follows:



For SI: 1 inch = 25.4 mm, 1 foot = 304.8 mm.

FIGURE R503.2.4

OPENING IN HORIZONTAL DIAPHRAGMS

Notes:

a. Blockings shall be provided beyond headers.

b. Metal ties not less than 0.058 inch [1.47 mm (16 galvanized gage)] by 1.5 inches (38 mm) wide with eight 16d common nails on each side of the header-joist intersection. The metal ties shall have a minimum yield of 33,000 psi (227 MPa).

c. Openings in diaphragms shall be further limited in accordance with Section R301.2.2.6.

SECTION 19. Table R602.3(1) is hereby amended to read as follows:

TABLE R602.3(1) -- FASTENING SCHEDULE

. . .

a. Nails are smooth-common, box or deformed shanks except where otherwise stated. Nails used for framing and sheathing connections are carbon steel and shall have minimum average bending yield strengths as shown: 80 ksi for shank diameter of 0.192 inch (20d common nail), 90 ksi for shank diameters larger than 0.142 inch but not larger than 0.177 inch, and 100 ksi for shank diameters of 0.142 inch or less. Connections using nails ~~and staples~~ of other materials, such as stainless steel, shall be designed by accepted engineering practice or approved under Section ~~R104.2.2~~104.2.8. Use of staples in roof, floor, subfloor, and braced wall panels shall be prohibited in Seismic Design Category D_0, D_1, or D_2.

. . .

SECTION 20. Table R602.3(2) is hereby amended to read as follows:

TABLE R602.3(2)

ALTERNATE ATTACHMENTS TO TABLE R602.3(1)

. . .

b. Staples shall have a minimum crown width of 7/16-inch except as noted. Use of staples in roof, floor, subfloor, and braced wall panels shall be prohibited in Seismic Design Category D_0, D_1, or D_2.

. . .

SECTION 21. Section R602.3.2 is hereby amended to read as follows:

R602.3.2 Top plate.

. . .

Exception: In other than Seismic Design Category D_0, D_1, or D_2, a~~A~~ single top plate used as an alternative to a double top plate shall comply with the following:

. . .

SECTION 22. Table R602.3.2 is hereby amended to read as follows:

TABLE R602.3.2
SINGLE TOP-PLATE SPLICE CONNECTION DETAILS

CONDITION	TOP-PLATE SPLICE LOCATION			
	Corners and intersecting walls		Butt joints in straight walls	
	Splice plate size	Minimum nails each side of joint	Splice plate size	Minimum nails each side of joint
~~Structures in SDC A, C, and in S-3C, D_0, D_1, and D_2 with braced wall line spacing less than 25 feet~~	~~3" × 6" × 0.036" galvanized steel plate or equivalent~~	~~(6) 8d box (2½" × 0.113") nails~~	~~3" × 12" × 0.036" galvanized steel plate or equivalent~~	~~(12) 8d box (2½" × 0.113") nails~~
~~Structures in SDC D_0, D_1, and D_2, with braced wall line spacing greater than or equal to 25 feet~~	~~3" × 8" by 0.036" galvanized steel plate or equivalent~~	~~(8) 8d box (2½" × 0.113") nails~~	~~5" × 18" × 0.036" galvanized steel plate or equivalent~~	~~(18) 8d box (2½" × 0.113") nails~~

~~For SI: 1 inch = 25.4 mm, 1 foot = 304.8 mm.~~

SECTION 23. Section R602.10.2.3 is hereby amended to read as follows:

R602.10.2.3 Minimum number of braced wall panels.

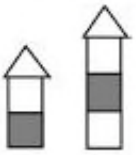
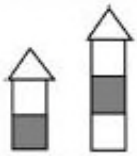
Braced wall lines with a length of 16 feet (4877 mm) or less shall have not less than two braced wall panels of any length or one braced wall panel equal to 48 inches (1219 mm) or more. Braced wall lines greater than 16 feet (4877 mm) shall have not less than two braced wall panels. In Seismic Design Category D_0, D_1, or D_2, no braced wall panel shall have a contributing length less than 48 inches in length or as required in Section R602.10.3, whichever is greater.

SECTION 24. Table R602.10.3(3) is hereby amended to read as follows:

TABLE R602.10.3(3)

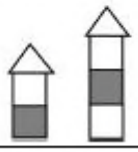
BRACING REQUIREMENTS BASED ON SEISMIC DESIGN CATEGORY

TABLE R602.10.3(3)
BRACING REQUIREMENTS BASED ON SEISMIC DESIGN CATEGORY

• WALL HEIGHT = 10 FEET • 10 PSF FLOOR DEAD LOAD • 15 PSF ROOF/CEILING DEAD LOAD • BRACED WALL LINE SPACING ≤ 25 FEET			MINIMUM TOTAL LENGTH (FEET) OF BRACED WALL PANELS REQUIRED ALONG EACH BRACED WALL LINE[a, g]				
Seismic Design Category[b]	Story Location	Braced Wall Line Length (feet)[c]	Method LIB[d]	Method GB[e]	Methods DWB, SFB, PBS, PCP, HPS, CS-SFB[e]	Methods WSP, ABW[f], PFH[f] and PFG[e, f]	Methods CS-WSP, CS-G, CS-PF
C (townhouses only)		10	2.5	2.5	2.5	1.6	1.4
		20	5.0	5.0	5.0	3.2	2.7
		30	7.5	7.5	7.5	4.8	4.1
		40	10.0	10.0	10.0	6.4	5.4
		50	12.5	12.5	12.5	8.0	6.8
		10	NP	4.5	4.5	3.0	2.6
		20	NP	9.0	9.0	6.0	5.1
		30	NP	13.5	13.5	9.0	7.7
		40	NP	18.0	18.0	12.0	10.2
		50	NP	22.5	22.5	15.0	12.8
		10	NP	6.0	6.0	4.5	3.8
		20	NP	12.0	12.0	9.0	7.7
		30	NP	18.0	18.0	13.5	11.5
		40	NP	24.0	24.0	18.0	15.3
		50	NP	30.0	30.0	22.5	19.1
D_0		10	NP	~~2.8~~ 5.6	~~2.8~~ 5.6	1.8	1.6
		20	NP	~~5.5~~11.0	~~5.5~~ 11.0	3.6	3.1
		30	NP	~~8.3~~ 16.6	~~8.3~~ 16.6	5.4	4.6
		40	NP	~~11.0~~ 22.0	~~11.0~~ 22.0	7.2	6.1
		50	NP	~~13.8~~ 27.6	~~13.8~~ 27.6	9.0	7.7
		10	NP	~~5.3~~ NP	~~5.3~~ NP	3.8	3.2
		20	NP	~~10.5~~ NP	~~10.5~~ NP	7.5	6.4
		30	NP	~~15.8~~ NP	~~15.8~~ NP	11.3	9.6
		40	NP	~~21.0~~ NP	~~21.0~~ NP	15.0	12.8
		50	NP	~~26.3~~ NP	~~26.3~~ NP	18.8	16.0
		10	NP	~~7.3~~ NP	~~7.3~~NP	5.3	4.5
		20	NP	~~14.5~~ NP	~~14.5~~ NP	10.5	9.0
		30	NP	~~21.8~~ NP	~~21.8~~ NP	15.8	13.4
		40	NP	~~29.0~~ NP	~~29.0~~ NP	21.0	17.9
		50	NP	~~36.3~~ NP	~~36.3~~ NP	26.3	22.3

(continued)

TABLE R602.10.3(3)—continued
BRACING REQUIREMENTS BASED ON SEISMIC DESIGN CATEGORY

• WALL HEIGHT = 10 FEET • 10 PSF FLOOR DEAD LOAD • 15 PSF ROOF/CEILING DEAD LOAD • BRACED WALL LINE SPACING ≤ 25 FEET			MINIMUM TOTAL LENGTH (FEET) OF BRACED WALL PANELS REQUIRED ALONG EACH BRACED WALL LINE[a, g]				
Seismic Design Category[h]	Story Location	Braced Wall Line Length (feet)[c]	Method LIB[d]	Method GB[d]	Methods DWB, SFB, PBS, PCP, HPS, CS-SFB[e]	Methods WSP, ABW[f], PFH[f] and PFG[e, f]	Methods CS-WSP, CS-G, CS-PF
D_1		10	NP	~~3.0~~ 6.0	~~3.0~~ 6.0	2.0	1.7
		20	NP	~~6.0~~ 12.0	~~6.0~~ 12.0	4.0	3.4
		30	NP	~~9.0~~ 18.0	~~9.0~~ 18.0	6.0	5.1
		40	NP	~~12.0~~ 24.0	~~12.0~~ 24.0	8.0	6.8
		50	NP	~~15.0~~ 30.0	~~15.0~~ 30.0	10.0	8.5
		10	NP	~~6.0~~ NP	~~6.0~~ NP	4.5	3.8
		20	NP	~~12.0~~ NP	~~12.0~~ NP	9.0	7.7
		30	NP	~~18.0~~ NP	~~18.0~~ NP	13.5	11.5
		40	NP	~~24.0~~ NP	~~24.0~~ NP	18.0	15.3
		50	NP	~~30.0~~ NP	~~30.0~~ NP	22.5	19.1
		10	NP	~~8.5~~ NP	~~8.5~~ NP	6.0	5.1
		20	NP	~~17.0~~ NP	~~17.0~~ NP	12.0	10.2
		30	NP	~~25.5~~ NP	~~25.5~~ NP	18.0	15.3
		40	NP	~~34.0~~ NP	~~34.0~~ NP	24.0	20.4
		50	NP	~~42.5~~ NP	~~42.5~~ NP	30.0	25.5
D_2[h]		10	NP	~~4.0~~ 8.0	~~4.0~~ 8.0	2.5	2.1
		20	NP	~~8.0~~ 16.0	~~8.0~~ 16.0	5.0	4.3
		30	NP	~~12.0~~ 24.0	~~12.0~~ 24.0	7.5	6.4
		40	NP	~~16.0~~ 32.0	~~16.0~~ 32.0	10.0	8.5
		50	NP	~~20.0~~ 40.0	~~20.0~~ 40.0	12.5	10.6
		10	NP	~~7.5~~ NP	~~7.5~~ NP	5.5	4.7
		20	NP	~~15.0~~ NP	~~15.0~~ NP	11.0	9.4
		30	NP	~~22.5~~ NP	~~22.5~~ NP	16.5	14.0
		40	NP	~~30.0~~ NP	~~30.0~~ NP	22.0	18.7
		50	NP	~~37.5~~ NP	~~37.5~~ NP	27.5	23.4
	Three-story dwelling	10	NP	NP	NP	NP	NP
		20	NP	NP	NP	NP	NP
		30	NP	NP	NP	NP	NP
		40	NP	NP	NP	NP	NP
		50	NP	NP	NP	NP	NP
	Cripple wall below one- or two-story dwelling	10	NP	NP	NP	7.5	6.4
		20	NP	NP	NP	15.0	12.8
		30	NP	NP	NP	22.5	19.1
		40	NP	NP	NP	30.0	25.5
		50	NP	NP	NP	37.5	31.9

(continued)

For SI: 1 inch – 25.4 mm, 1 foot = 304.8 mm, 1 pound per square foot = 0.0479 kPa.

NP – Not Permitted

. . .

i. Methods GB and PCP braced wall panel h/w ratio shall not exceed 1:1 in SDC D_0, D_1 and D_2.

Methods DWB, SFB, PBS, and HPS are not permitted in D_0, D_1 or D_2.

SECTION 25. Table R602.10.4 is hereby amended to read as follows:

TABLE R602.10.4

BRACING METHODS

TABLE R602.10.4
BRACING METHODS [1]

	METHODS, MATERIAL	MINIMUM THICKNESS	FIGURE	CONNECTION CRITERIA[2]	
				Fasteners	Spacing
Intermittent Bracing Methods	**LIB** Let-in-bracing	1 × 4 wood or approved metal straps at 45° to 60° angles for maximum 16″ stud spacing		Wood: 2-8d common nails or 3-8d ($2^1/_2$″ long x 0.113″ dia.) nails	Wood: per stud and top and bottom plates
				Metal strap: per manufacturer	Metal: per manufacturer
	DWB Diagonal wood boards	$^3/_4$″ (1″ nominal) for maximum 24″ stud spacing		2-8d ($2^1/_2$″ long × 0.113″ dia.) nails or 2 - $1^3/_4$″ long staples	Per stud
	WSP Wood structural panel (See Section R604)	~~$^3/_8$″~~ 15/32″	8d common (2 1/2″x0.131) nails 3/8″ ~~edge distance~~ to panel edge	~~Exterior sheathing per Table R602.3(3)~~	6″ edges 12″ field
			8d common (2 1/2″x0.131) nails 3/8″ edge distance to panel edge	~~Interior sheathing per Table R602.3(1) or R602.3(2)~~	~~Varies by fastener~~ 6″ edges 12″ field
	BV-WSP[4] Wood structural panels with stone or masonry veneer (See Section R602.10.6.5)	$^7/_{16}$″	See Figure R602.10.6.5	8d common ($2^1/_2$″ × 0.131) nails	4″ at panel edges 12″ at intermediate supports 4″ at braced wall panel end posts
	SFB Structural fiberboard sheathing	$^1/_2$″ or $^{25}/_{32}$″ for maximum 16″ stud spacing		$1^1/_2$″ long × 0.12″ dia. (for $^1/_2$″ thick sheathing) $1^3/_4$″ long × 0.12″ dia. (for $^{25}/_{32}$″ thick sheathing) galvanized roofing nails	3″ edges 6″ field
	GB Gypsum board	$^1/_2$″		Nails or screws per Table R602.3(1) for exterior locations	For all braced wall panel locations: 7″ edges (including top and bottom plates) 7″ field
				Nails or screws per Table R702.3.5 for interior locations	
	PBS Particleboard sheathing (See Section R605)	$^3/_8$″ or $^1/_2$″ for maximum 16″ stud spacing		For $^3/_8$″, 6d common (2″ long × 0.113″ dia.) nails For $^1/_2$″, 8d common ($2^1/_2$″ long × 0.131″ dia.) nails	3″ edges 6″ field
	PCP Portland cement plaster	See Section R703.7 for maximum 16″ stud spacing		$1^1/_2$″ long, 11 gage, 0.120″ dia., $^7/_{16}$″ dia. head nails or $^7/_8$″ long, 16 gage staples	6″ o.c. on all framing members
	HPS Hardboard panel siding	$^7/_{16}$″ for maximum 16″ stud spacing		0.092″ dia., 0.225″ dia. head nails with length to accommodate $1^1/_2$″ penetration into studs	4″ edges 8″ field
	ABW Alternate braced wall	$^3/_8$″		See Section R602.10.6.1	See Section R602.10.6.1

(continued)

TABLE R602.10.4—continued
BRACING METHODS [f]

METHODS, MATERIAL		MINIMUM THICKNESS	FIGURE	CONNECTION CRITERIA[a]	
				Fasteners	Spacing
Intermittent Bracing Methods	**PFH** Portal frame with hold-downs	$^3/_8$"		See Section R602.10.6.2	See Section R602.10.6.2
	PFG Portal frame at garage	$^7/_{16}$"		See Section R602.10.6.3	See Section R602.10.6.3
Continuous Sheathing Methods	**CS-WSP** Continuously sheathed wood structural panel	$^3/_8$"	8d common (2 1/2"x0.131) nails 3/8" edge distance to panel edge	~~Exterior sheathing per Table R602.3(3)~~	6" edges 12" field
		15/32"	8d common (2 1/2"x0.131) nails 3/8" edge distance to panel edge	~~Interior sheathing per Table R602.3(1) or R602.3(2)~~	~~Varies by fastener~~ 6" edges 12" field
	CS-G[b,c] Continuously sheathed wood structural panel adjacent to garage openings	$^3/_8$" 15/32"		See Method CS-WSP	See Method CS-WSP
	CS-PF Continuously sheathed portal frame	$^7/_{16}$" 15/32"		See Section R602.10.6.4	See Section R602.10.6.4
	CS-SFB[d] [f] Continuously sheathed structural fiberboard	$^1/_2$" or $^{25}/_{32}$" for maximum 16" stud spacing		$1^1/_2$" long × 0.12" dia. (for $^1/_2$" thick sheathing) $1^3/_4$" long × 0.12" dia. (for $^{25}/_{32}$" thick sheathing) galvanized roofing nails	3" edges 6" field

For SI: 1 inch = 25.4 mm, 1 foot = 304.8 mm, 1 degree = 0.0175 rad, 1 pound per square foot = 47.8 N/m^2, 1 mile per hour = 0.447 m/s.

a. Adhesive attachment of wall sheathing, including Method CS, shall not be permitted in Seismic Design Categories D_1, D_2, D_3 and D_4.

b. Applies to panels next to garage door opening where supporting gable end wall or roof load only. Shall only be used on one wall of the garage. In Seismic Design Categories D_1, D_2 and D_3, roof covering dead load shall not exceed 3 psf.

c. Garage openings adjacent to a Method CS-G panel shall be provided with a header in accordance with Table R602.5(1). A full height clear opening shall not be permitted adjacent to a Method CS-G panel.

d. Method CS-SFB does not apply in Seismic Design Categories D_1, D_2 and D_3.

e. Method applies to detached one- and two-family dwellings in Seismic Design Categories D_0 through D_2 only.

f. Methods GB and PCP braced wall panel bracing shall not exceed 14 ft in SDC D_0, D_1, or D_2. Methods LIB, DWB, SFB, PBS, HPS, and PPC are not permitted in SDC D_0, D_1, or D_2.

g. Use of staples in braced wall panels shall be prohibited in SDC D_0, D_1, or D_2.

SECTION 26. Table R602.10.5 is hereby amended to read as follows:

TABLE R602.10.5

MINIMUM LENGTH OF BRACED WALL PANELS

TABLE R602.10.5
MINIMUM LENGTH OF BRACED WALL PANELS

METHOD (See Table R602.10.4)		MINIMUM LENGTH[a] (inches) Wall Height					CONTRIBUTING LENGTH (inches)
		8 feet	9 feet	10 feet	11 feet	12 feet	
DWB, WSP, SFB, PBS, PCP, HPS, BV-WSP		48	48	48	53	58	Actual[b]
GB		48	48	48	53	58	Double sided = Actual Single sided = 0.5 × Actual
LIB		55	62	69	NP	NP	Actual[b]
ABW	SDC A, B and C, ultimate design wind speed < 140 mph	28	32	34	38	42	48
	SDC D_0, D_1 and D_2, ultimate design wind speed < 140 mph	32	32	34	NP	NP	
CS-G		24	27	30	33	36	Actual[b]
CS-WSP, CS-SFB	Adjacent clear opening height (inches)						
	≤ 64	24	27	30	33	36	Actual[b]
	68	26	27	30	33	36	
	72	27	27	30	33	36	
	76	30	29	30	33	36	
	80	32	30	30	33	36	
	84	35	32	32	33	36	
	88	38	35	33	33	36	
	92	43	37	35	35	36	
	96	48	41	38	36	36	
	100	—	44	40	38	38	
	104	—	49	43	40	39	
	108	—	54	46	43	41	
	112	—	—	50	45	43	
	116	—	—	55	48	45	
	120	—	—	60	52	48	
	124	—	—	—	56	51	
	128	—	—	—	61	54	
	132	—	—	—	66	58	
	136	—	—	—	—	62	
	140	—	—	—	—	66	
	144	—	—	—	—	72	
METHOD (See Table R602.10.4)		Portal header height 8 feet	9 feet	10 feet	11 feet	12 feet	
PFH	Supporting roof only	~~16~~ 24	~~16~~ 24	~~16~~ 24	Note c	Note c	48
	Supporting one story and roof	24	24	24	Note c	Note c	
PFG		24	27	30	Note d	Note d	1.5 × Actual[b]
CS-PF	SDC A, B and C	16	18	20	Note e	Note e	1.5 × Actual[b]
	SDC D_0, D_1 and D_2	~~16~~ 24	~~18~~ 24	~~20~~ 24	Note e	Note e	Actual[b]

For SI: 1 inch = 25.4 mm, 1 foot = 304.8 mm, 1 mile per hour = 0.447 m/s.
NP = Not Permitted.

. . .

SECTION 27. Figure R602.10.6.1 is amended to read as follows:

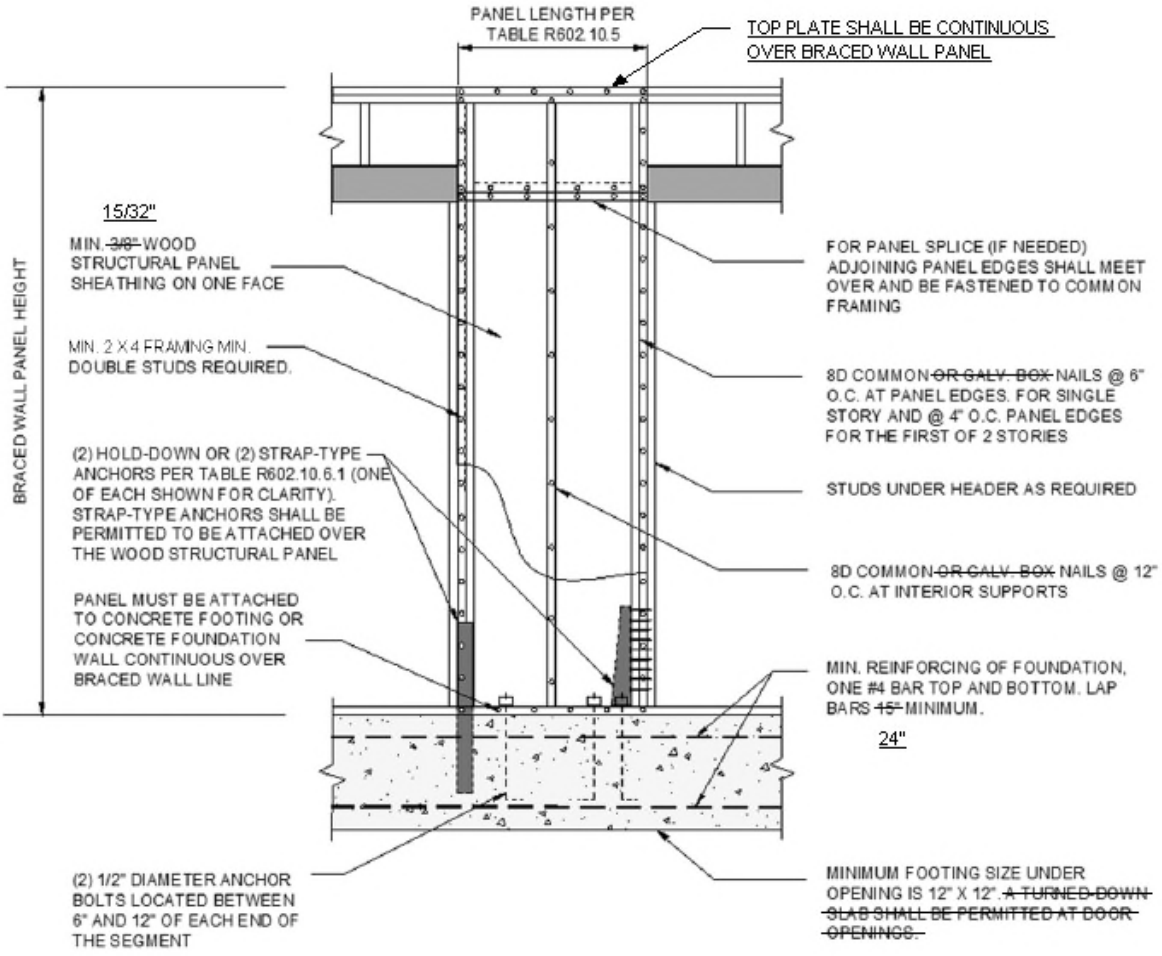
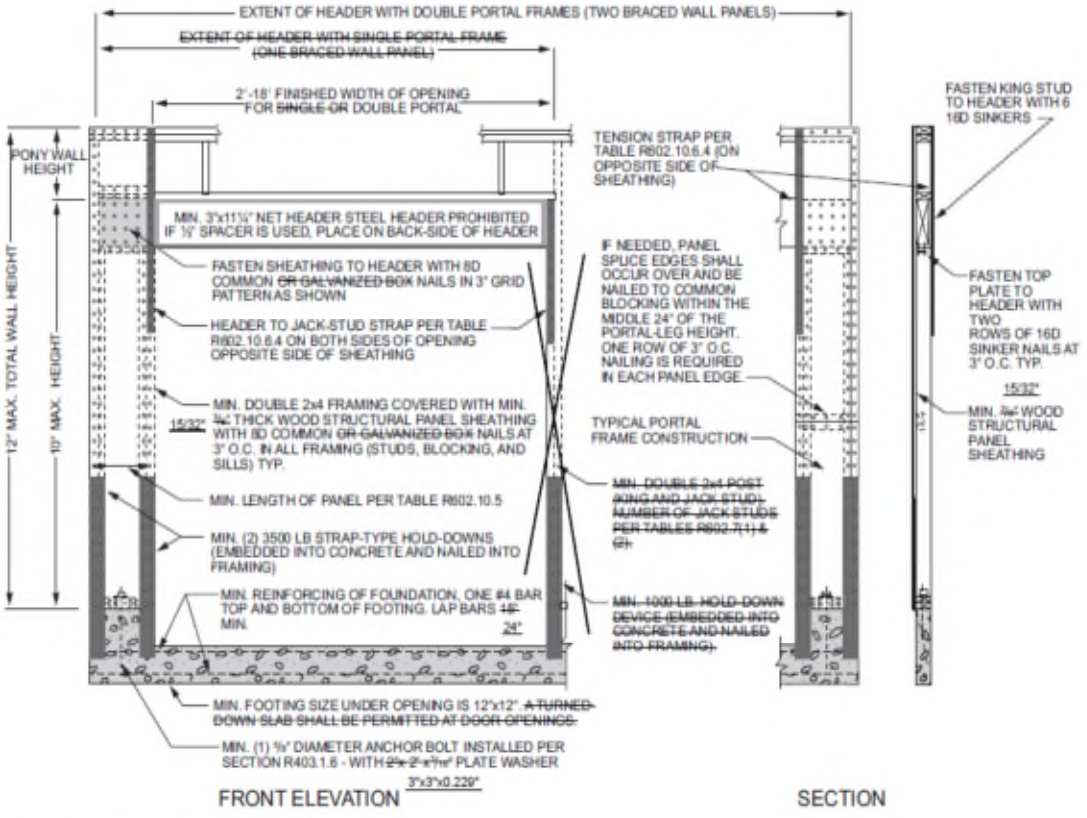


FIGURE R602.10.6.1
METHOD ABW—ALTERNATE BRACED WALL PANEL

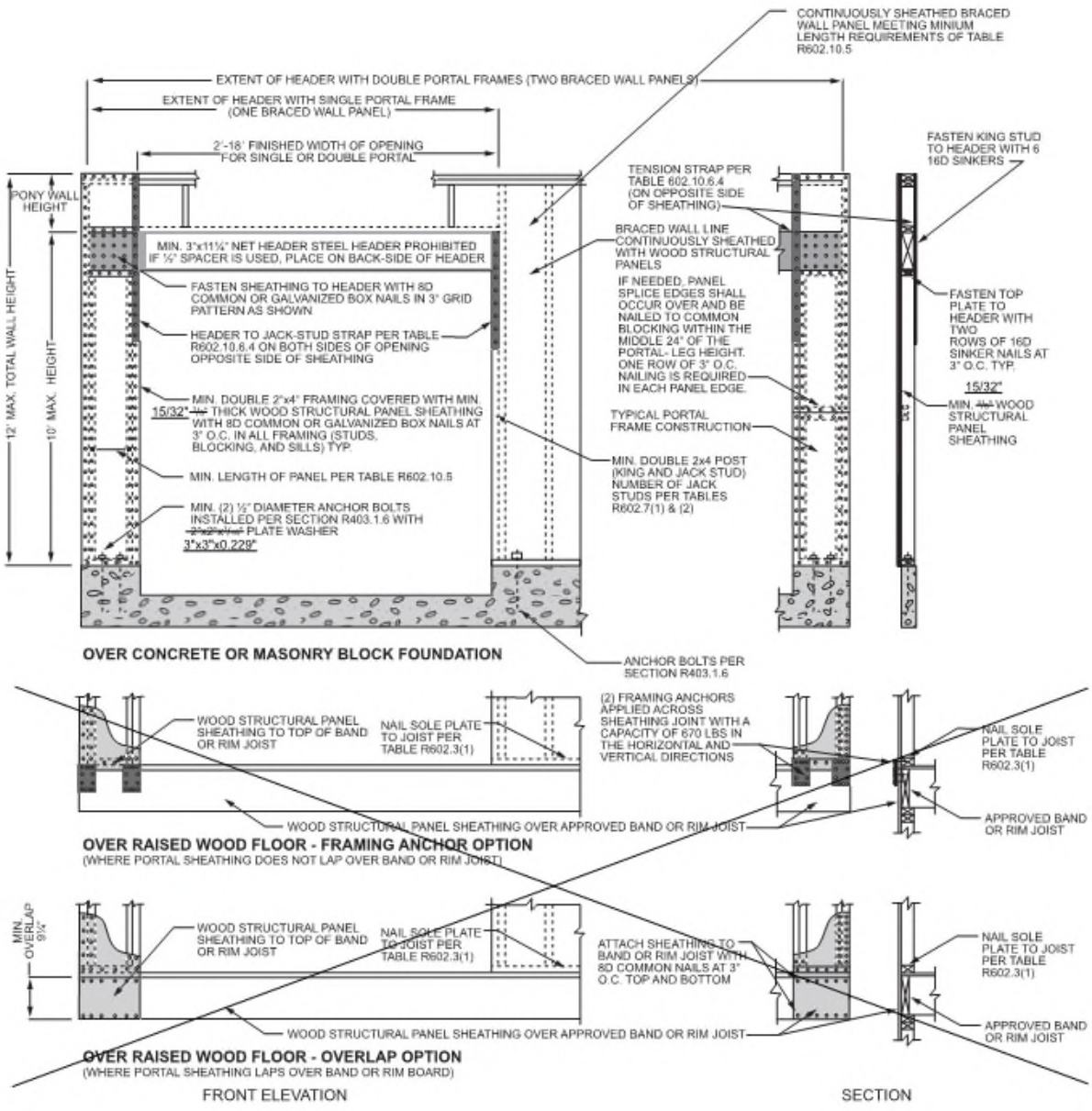
SECTION 28. Figure R602.10.6.2 is hereby amended to read as follows:



For SI: 1 inch = 25.4 mm, 1 foot = 304.8 mm.

FIGURE R602.10.6.2
METHOD PFH—PORTAL FRAME WITH HOLD-DOWNS
AT DETACHED GARAGE DOOR OPENINGS

SECTION 29. Figure R602.10.6.4 is hereby amended to read as follows:



For SI: 1 inch = 25.4 mm, 1 foot = 304.8 mm.

FIGURE R602.10.6.4
METHOD CS-PF—CONTINUOUSLY SHEATHED PORTAL FRAME PANEL CONSTRUCTION

SECTION 30. Section R606.4.4 is hereby amended to read as follows:

R606.4.4 Parapet walls.

Unreinforced solid masonry parapet walls shall not be less than 8 inches (203 mm) thick and their height shall not exceed four times their thickness. Unreinforced hollow unit masonry parapet walls shall be not less than 8 inches (203 mm) thick, and their height shall not exceed three times their thickness. Masonry parapet walls in areas subject to wind loads of 30 pounds per square foot (1.44 kPa), or located in Seismic Design Category D_0, D_1, or D_2, or on townhouses in Seismic Design Category C shall be reinforced in accordance with Section R606.12.

SECTION 31. Section R606.12.2.2.3 is hereby amended to read as follows:

R606.12.2.2.3 Reinforcement requirements for masonry elements.

Masonry elements listed in Section R606.12.2.2.2 shall be reinforced in either the horizontal or vertical direction as shown in Figure R606.11(2) and in accordance with the following:

1. Horizontal reinforcement. Horizontal joint reinforcement shall consist of ~~not less than two longitudinal W1.7 wires spaced not more than 16 inches (406 mm) for walls greater than 4 inches (102 mm) in width and not less than one longitudinal W1.7 wire spaced not more than 16 inches (406 mm) for walls not exceeding 4 inches (102 mm) in width; or~~ not less than one No. 4 bar spaced not more than 48 inches (1219 mm). ~~Where two longitudinal wires of joint reinforcement are used, the space between these wires shall be the widest that the mortar joint will accommodate.~~

Horizontal reinforcement shall be provided within 16 inches (406 mm) of the top and bottom of these masonry elements.

2. Vertical reinforcement. Vertical reinforcement shall consist of not less than one No. 4 bar spaced not more than 48 inches (1219 mm). Vertical reinforcement shall be located within ~~16~~8 inches (~~406~~203 mm) of the ends of masonry walls.

SECTION 32. Section R803.2.4 is hereby added to read as follows:

R803.2.4 Openings in horizontal diaphragms.

Openings in horizontal diaphragms shall conform with Section R503.2.4.

SECTION 33. Section R1001.3.1 is hereby amended to read as follows:

R1001.3.1 Vertical reinforcing.

For chimneys up to 40 inches (1016 mm) wide, four No. 4 continuous vertical bars adequately anchored into the concrete foundation shall be placed between wythes of solid masonry or within the cells of hollow unit masonry and grouted in accordance with Section R606. Grout shall be prevented from bonding with the flue liner so that the flue liner is free to move with thermal expansion. For chimneys more than 40 inches (1016 mm) wide, two additional No. 4 vertical bars adequately anchored into the concrete foundation shall be provided for each additional flue incorporated into the chimney or for each additional 40 inches (1016 mm) in width or fraction thereof.

SECTION 34. Section BJ106.1 is hereby amended to read as follows:

BJ106.1 General. In other than Seismic Design Category D_0, D_1, D_2, E, or F, p~~P~~lastered strawbale walls shall be permitted to be used as structural walls in accordance with the prescriptive provisions of this s~~S~~ection.

SECTION 35. Section CJ101.1 is hereby amended to read as follows:

CJ101.1 Scope.

This appendix ~~shall be applicable~~applies to emergency housing and emergency housing facilities, as defined in Section CJ102, when and to the extent that the County of Los Angeles Board of Supervisors ("Board") finds, by motion, resolution, or otherwise, that this appendix applies to a specific state of emergency, local emergency, or declaration of shelter crisis.

SECTION 36. Section CJ102.1 is hereby amended to read as follows:

CJ102.1 General.

. . .

ENFORCING AGENCY. The Building Official as defined in Section 104.3 of this Code.

. . .

SECTION 37. Section CJ103.1 is hereby amended to read as follows:

CJ103.1 General.

Emergency sleeping cabins, emergency transportable housing units, membrane structures and tents constructed and/or assembled in accordance with this appendix, shall be occupied only during the duration of the declaration of state of emergency, local emergency, or shelter crisis.

. . .

SECTION 38. Section CJ103.4 is hereby amended to read as follows:

CJ103.4 Fire and life safety requirements not addressed in this appendix.

If not otherwise addressed in this appendix, fire and life safety measures, including, but not limited to, means of egress, fire separation, fire sprinklers, smoke alarms, and carbon monoxide alarms, shall be determined and enforced by the enforcing agency in consultation with the Departments of Public Health, Fire, and other pertinent County departments, as applicable.

SECTION 39. Section CJ106.1 is hereby amended to read as follows:

CJ106.1 General.

. . .

Tents and membrane structures shall be provided with means of ventilation (natural and/or mechanical) allowing for adequate air replacement, as determined by the enforcing agency.

SECTION 40. Section CJ107.1 is hereby amended to read as follows:

CJ107.1 General.

Emergency housing shall comply with the applicable requirements in the ~~California~~ Building Code, Chapter 11B and/or the US Access Board Final Guidelines for Emergency Transportable Housing as determined by the enforcing agency.

. . .

SECTION 41. Section CJ110.1.1 is hereby added to read as follows:

CJ110.1.1 Backflow prevention.

Backflow prevention devices shall be provided in accordance with Section 602.3 of the Plumbing Code.

SECTION 42. Section CJ110.1.2 is hereby added to read as follows:

CJ110.1.2 Drinking fountains.

An adequate number of drinking fountains, bottle fillers or drinking facilities shall be provided as determined by the enforcing agency.

SECTION 43. Section CJ110.3 is hereby amended to read as follows:

CJ110.3 Toilet and bathing facilities.

. . .

The maximum travel distance from any sleeping and/or living area to the toilet facility shall not exceed 300 feet (91.4 m) or as determined by the enforcing agency.

SECTION 44. The provisions of this ordinance contain various changes, modifications, and additions to the 2025 Edition of the California Residential Code. Some of these changes are administrative in nature in that they do not constitute changes or modifications to requirements contained in the building standards published in the California Building Standards Code.

Pursuant to California Health and Safety Code sections 17958.5, 17958.7, and 18941.5, the Board of Supervisors hereby expressly finds that all of the changes and modifications to requirements contained in the building standards published in the California Building Standards Code contained in this ordinance are reasonably necessary because of local climatic, geological, or topographical conditions in the

County of Los Angeles due to the potential for seismic activity in the region, topographical conditions that contribute to the spread of wild fires, and climatic conditions that impact air quality and increase the risk of wild fires. In addition, the Board of Supervisors expressly finds that the modifications herein are substantially equivalent to modifications that were previously filed by the County of Los Angeles and were in effect as of September 30, 2025. Without limiting the foregoing, the County makes additional findings herein:

Code Section	Condition	Explanation of Amendment
R301.1.3.2	Geological	Los Angeles County is prone to seismic activity due to the existence of active faults in the Southern California area. After the 1994 Northridge Earthquake, the Wood Frame Construction Joint Task Force recommended that the quality of woodframe construction needed to be greatly improved. The Task Force recommended that structural plans be prepared by the engineer or architect so that plan examiners, building inspectors, contractors, and special inspectors may logically follow and construct the seismic force-resisting systems as presented in the construction documents. For buildings or structures located in Seismic Design Category D_0, D_1, D_2, or E that are subject to a greater level of seismic forces, the requirement to have a California licensed architect or engineer prepare the construction documents is intended to minimize or reduce structural deficiencies that may cause excessive damage or injuries in woodframe buildings. Involvement of a registered professional will minimize the occurrence of structural deficiencies such as plan and vertical irregularities, improper shear transfer of the seismic force-resisting system, missed details or connections important to the structural system, and the improper application of the prescriptive requirements of the California Residential Code.

R301.1.5	Geological Topographical	Due to the local topographical and geological conditions of the sites within the greater Los Angeles region and their susceptibility to earthquakes, this technical amendment is required to address and clarify special needs for buildings constructed on hillside locations. A joint Structural Engineers Association of Southern California (SEAOSC) and Los Angeles City Joint Task Force investigated the performance of hillside building failures after the Northridge Earthquake. Numerous hillside failures resulted in loss of life and millions of dollars in damage. These criteria were developed to minimize the damage to these structures and have been in use by the City and County of Los Angeles for several years.
R301.2.2.6	Geological	Los Angeles County is prone to seismic activity due to the existence of active faults in the Southern California area. Due to the high geologic activities in the Southern California area and the necessary higher level of performance required for buildings and structures, this local amendment limits the type of irregular conditions as specified in the California Residential Code. Such limitations are recommended to reduce structural damage in the event of an earthquake. The County of Los Angeles and cities in this region have implemented these extra measures to maintain the structural integrity of the framing of the shear walls and all associated elements when designed for high levels of seismic loads.
R301.2.2.11	Geological	Los Angeles County is prone to seismic activity due to the existence of active faults in the Southern California area. Due to the high geologic activity in the Southern California area and the necessary higher level of performance required for buildings and structures, this local amendment limits the potential anchorage and supporting frame failure resulting from additional weight. There is no limitation for weight of mechanical and plumbing fixtures and equipment in the International Residential Code. Requirements from ASCE 7 and the International Building Code would permit equipment weighing up to 400 lbs. when mounted at 4 feet or less above the floor or attic level without engineering design. Where equipment exceeds this requirement, it is the intent of this

		amendment that a registered design professional be required to analyze if the floor support is adequate and structurally sound.
Table R302.1(2)	Climatic	This amendment will not allow unprotected openings (openings that do not resist the spread of fire) to be in the exterior wall of a residential building that is located on a property line. This amendment is necessary due to local climatic conditions. The hot, dry weather conditions of late summer in combination with the Santa Ana winds creates an extreme fire danger. Residential buildings with unprotected openings located on a property line may permit fires to spread from the inside of the building to adjacent properties and likewise from exterior properties to the interior of the building.
R337.1.1 R337.1.2	Climatic	Clarifies the application of Chapter 5 of the California Wildland Interface Code to include additions, alterations, and/or relocated buildings. Many areas of the County have been designated as Fire Hazard Severity Zones due to the increased risk of fire caused by low humidity, strong winds, and dry vegetation. Additions, alterations, and/or relocated buildings have the same fire risk as new buildings.
R337.1.3 R337.1.5 R337.1.6	Climatic	Disallows the use of wood-shingle/wood-shake roofs due to the increased risk of fire in the County caused by low humidity, strong winds, and dry vegetation in high fire severity zones.
R337.1.7	Climatic	Disallows the use of Class B wood-shingle/wood-shake roofs due to the increased risk of fire in the County caused by low humidity, strong winds, and dry vegetation in high fire severity zones.

R401.1	Geological	Los Angeles County is prone to seismic activity due to the existence of active faults in the Southern California area. Wood foundations, even those that are preservative-treated, encounter a higher risk of deterioration when contacting the adjacent ground. The required seismic anchorage and transfer of lateral forces into the foundation system necessary for 2-story structures and foundation walls could become compromised at varying states of wood decay. In addition, global structure overturning moment and sliding resistance is reduced when utilizing wood foundations as opposed to conventional concrete or masonry systems. However, non-occupied, single-story storage structures pose significantly less risk to human safety and may utilize the wood foundation guidelines specified in this Chapter.

R403.1.2 R403.1.3.6 R403.1.5 Figure R403.1.5	Climatic Geological	Los Angeles County is prone to seismic activity due to the existence of active faults in the Southern California area. These amendments require minimum reinforcement in continuous footings and stepped footings to address the problem of poor performance of plain or under-reinforced footings during a seismic event. These amendments implement the recommendations of SEAOSC and the Los Angeles City Joint Task Force resulting from their investigation of the 1994 Northridge Earthquake. Interior walls can easily be called upon to resist over half of the seismic loading imposed on simple buildings or structures. Without a continuous foundation to support the braced wall line, seismic loads would be transferred through other elements such as non-structural concrete slab floors, wood floors, etc. Requiring interior braced walls to be supported by continuous foundations is intended to reduce or eliminate the poor performance of buildings or structures.
R404.2	Climatic Geological	No substantiating data has been provided to show that wood foundations are effective in supporting structures and buildings during a seismic event while being subject to deterioration caused by the presence of water and other materials detrimental to wood foundations in the soil. Wood foundations, when they are not properly treated and protected against deterioration, have performed very poorly and have led to slope failures. Most contractors are typically accustomed to construction in dry weather in the Southern California region and are not generally familiar with the necessary precautions and treatment of wood that makes it suitable for both seismic events and wet applications. With the higher seismic demand placed on buildings and structures in this region, coupled with the dryer weather conditions, it is the intent of this amendment to reduce or eliminate potential problems resulting from the use of wood footings and foundations.

R501.2	Geological	Due to the high geologic activities in the Southern California area and the necessary higher level of performance required for buildings and structures, this local amendment limits the potential anchorage and supporting frame failure resulting from additional weight. There is no limitation for weight of mechanical and plumbing fixtures and equipment in the International Residential Code. Requirements from ASCE 7 and the International Building Code would permit equipment weighing up to 400 lbs. when mounted at 4 feet or less above the floor or attic level without engineering design. Where equipment exceeds this requirement, it is the intent of this amendment that a registered design professional be required to analyze if the floor support is adequate and structurally sound.
R503.2.4 Figure R503.2.4	Geological	Section R502.10 of the Code does not provide any prescriptive criteria to limit the maximum floor opening size, nor does Section R503 provide any details to address the issue of shear transfer near larger floor openings. With the higher seismic demand placed on buildings and structures in this region, it is important to ensure that a complete load path is provided to reduce or eliminate potential damage caused by seismic forces. Requiring blocking with metal ties around larger floor openings and limiting opening size is consistent with the requirements of Section R301.2.2.6.
Table R602.3(1) Table R602.3(2)	Geological	Los Angeles County is prone to seismic activity due to the existence of active faults in the Southern California area. In September 2007, limited cyclic testing data was provided to the ICC Los Angeles Chapter Structural Code Committee showing that stapled wood structural shear panels do not exhibit the same behavior as the nailed wood structural shear panels. The test results of the stapled wood structural shear panels demonstrated lower strength and drift than the nailed wood structural shear panel test results. Therefore, the use of staples as fasteners for shear walls sheathed with other materials shall not be permitted without being substantiated by cyclic testing.

R602.3.2 Table R602.3.2	Geological	Los Angeles County is prone to seismic activity due to the existence of active faults in the Southern California area. The County of Los Angeles and cities in this region have taken extra measures to maintain the structural integrity of the framing of the shear walls when designed for high levels of seismic loads by eliminating single top plate construction. The performance of modern day braced wall panel construction is directly related to an adequate load path extending from the roof diaphragm to the foundation system.
R602.10.2.3	Geological	The greater Los Angeles region is a densely populated area having buildings and structures constructed over and near a vast array of fault systems capable of producing major earthquakes, including, but not limited, to the 1994 Northridge Earthquake. Plywood shear walls with high aspect ratio experienced many failures during the Northridge Earthquake. This proposed amendment specifies a minimum braced wall length to meet an aspect ratio consistent with other sections of the California Residential Code, and to assure that new buildings and additions to existing buildings are designed and constructed in accordance with the scope and objectives of the California Residential Code. This is intended to improve the performance level of buildings and structures that are subject to the higher seismic demands and reduce and limit potential damage to property. This proposed amendment reflects the recommendations by SEAOSC and the Los Angeles City Joint Task Force that investigated the poor performance observed during the 1994 Northridge Earthquake.

Table R602.10.3(3)	Geological	Due to the high geologic activities in the Southern California area and the necessary higher level of performance of buildings and structures, this local amendment reduces or eliminates the allowable shear values for shear walls sheathed with lath, plaster, or gypsum board. The poor performance of such shear walls sheathed with other materials in the 1994 Northridge Earthquake was investigated by SEAOSC and the Los Angeles City Joint Task Force. The County of Los Angeles and cities in this region have taken extra measures to maintain the structural integrity of the framing of the shear walls when designed for high levels of seismic loads.
Table R602.10.4	Geological	3/8" thick 3 ply-plywood shear walls experienced many failures during the Northridge Earthquake. This amendment specifies minimum WSP sheathing thickness and nail size and spacing, so as to provide a uniform standard of construction to improve the performance level of buildings and structures, given the potential for higher seismic demands placed on buildings or structure in this region. This proposed amendment reflects the recommendations by SEAOSC and the Los Angeles City Joint Task Force following the 1994 Northridge Earthquake. In September 2007, cyclic testing data was provided to the Los Angeles Chapter Structural Code Committee showing that stapled wood structural shear panels underperformed nailed wood structural shear panels. Test results of the stapled wood structural shear panels appeared much lower in strength and drift than the nailed wood structural shear panel test results.

Table R602.10.5	Geological	Los Angeles County is prone to seismic activity due to the existence of active faults in the Southern California area. The poor performance of such shear walls sheathed in the 1994 Northridge Earthquake was investigated by SEAOSC and the Los Angeles City Joint Task Force. The County of Los Angeles and cities in this region have taken extra measures to maintain the structural integrity with respect to the "maximum shear wall aspect ratios" of the framing of the shear walls when designed for high levels of seismic loads. This amendment is consistent with the shear wall aspect ratio provision of Section 4.3.3 of AWC SDPWS-2021.
Figure R602.10.6.1	Geological	3/8" thick 3 ply-plywood shear walls experienced many failures during the Northridge Earthquake. The poor performance of shear walls in the 1994 Northridge Earthquake was investigated by SEAOSC and the Los Angeles City Joint Task Force. Box nails were observed to cause massive and multiple failures of the typical 3/8" thick 3 ply-plywood during the Northridge Earthquake. The County of Los Angeles and cities in this region have taken extra measures to maintain the structural integrity of the framing of the shear walls when designed for high levels of seismic loads. The performance of modern day braced wall panel construction is directly related to an adequate load path extending from the roof diaphragm to the foundation system.
Figure R602.10.6.2	Geological	3/8" thick 3 ply-plywood shear walls experienced many failures during the Northridge Earthquake. The poor performance of such shear walls in the 1994 Northridge Earthquake was investigated by SEAOSC and the Los Angeles City Joint Task Force. The County of Los Angeles and cities in this region have taken extra measures to maintain the structural integrity of the framing of the shear walls when designed for high levels of seismic loads. Box nails were observed to cause massive and multiple failures of typical 3/8-inch thick plywood during the Northridge Earthquake. This change to the minimum lap splice requirement is consistent with Section 25.5 of ACI 318-19.
Figure R602.10.6.4	Geological	3/8" thick 3 ply-plywood shear walls experienced many failures during the Northridge Earthquake. The

		poor performance of such shear walls in the 1994 Northridge Earthquake was investigated by SEAOSC and the Los Angeles City Joint Task Force. The County of Los Angeles and cities in this region have taken extra measures to maintain the structural integrity of the framing of the shear walls when designed for high levels of seismic loads. The proposal in which "washers shall be a minimum of 0.229 inch by 3 inches by 3 inches in size" is consistent with Section R602.11.1 of the California Residential Code and Section 2308.7.1 of the California Building Code.
R606.4.4	Geological	Los Angeles County is prone to seismic activity due to the existence of active faults in the Southern California area. The addition of the word "or" will prevent the use of unreinforced parapets in Seismic Design Category D_0, D_1, or D_2, or on townhouses in Seismic Design Category C.
R606.12.2.2.3	Geological	Los Angeles County is prone to seismic activity due to the existence of active faults in the Southern California area. Reinforcement using longitudinal wires for buildings and structures located in high seismic areas is not as ductile as deformed rebar. Having vertical reinforcement closer to the ends of masonry walls helps to improve the seismic performance of masonry buildings and structures.
R803.2.4	Geological	Section R802 of the Code does not provide any prescriptive criteria to limit the maximum size of roof openings, nor does Section R803 provide any details to address the issue of shear transfer near larger roof openings. With the higher seismic demand placed on buildings and structures in this region, it is important to ensure that a complete load path is provided to reduce or eliminate potential damage caused by seismic forces. Requiring blocking with metal ties around larger roof openings and limiting the size of openings is consistent with the requirements of Section R301.2.2.6.

R1001.3.1	Geological	Los Angeles County is prone to seismic activity due to the existence of active faults in the Southern California area. The performance of fireplaces/chimneys without anchorage to the foundation has been observed to be inadequate during major earthquakes. The lack of anchorage to the foundation results in overturn or displacement.
Appendix BJ BJ106.1	Geological	Los Angeles County is prone to seismic activity due to the existence of active faults in the Southern California area. Due to the limited seismic performance information on strawbale construction, this amendment is intended to limit the higher risk strawbale construction poses in a high seismic region.
Appendix CJ CJ101.1, CJ102.1, CJ103.1, CJ103.4, CJ107.1	Administrative, Climatic, Geologic, Topographical	Adoption of this appendix is necessary because strict compliance with state and local standards and laws would prevent, hinder, or delay the mitigation of the effects of a declared shelter crisis or other emergency. The modifications to this appendix are administrative in nature, to provide clarification of various provisions of the language of this voluntary Appendix.
CJ106.1	Climatic	Los Angeles County is subject to extreme temperatures, and many of these membrane structures will be erected and occupied during severe weather events. It is necessary to include this amendment to ensure the safety, health, and comfort of the occupants is maintained during extreme heat and cold.
CJ110.1.1, CJ110.1.2	Administrative	These sections are simply a cross reference to the State Plumbing Code requirement for user convenience and is not adding a new building standard nor enacting a more restrictive requirement. To the extent findings are requested, see prefatory language in this Section.
CJ110.3	Climatic	The County may utilize mobile restroom facilities that are physically separate from the living facilities. Due to the potential for severe local weather conditions, with extreme temperatures or torrential rain, the distance to the restroom facilities required for the comfort, safety, and health of displaced people should be reduced to 300 feet or as determined by the Building Official.

SECTION 45. This ordinance shall become operative on January 1, 2026.

[TITLE30MACOCO]